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EVALUATION SUBJECT: USP® STRUCTURAL CONNECTORS FOR WOOD-FRAMED CONSTRUCTION

REPORT HOLDER:

Mitek USA, Inc. 14305 Southcross Drive, Suite 200 Burnsville, Minnesota 55306 (952) 898-8772 www.uspconnectors.com

CSI Division:

06-WOOD AND PLASTICS

CSI Section:

06 05 23–Wood, Plastic and Composite Fastenings

1.0 SCOPE OF EVALUATION

1.1 Compliance to the following codes & regulations:

- 2015, 2012, 2009, 2006 International Building Code[®] (IBC)
- 2015, 2012, 2009, 2006 International Residential Code[®] (IRC)

1.2 Evaluated in accordance with:

- IAPMO UES Evaluation Criteria for Joist Hangers and Miscellaneous Connectors (EC 002-2017)
- ICC-ES AC13, approved February 2017

1.3 Properties assessed:

Structural

2.0 PRODUCT USE

The MiTek USP[®] structural connectors described in this report are used for connecting wood framing members in accordance with Section 2304.10.3 of the 2015 IBC, or Section 2304.9 of the 2012, 2009 and 2006 IBC. These devices may also be used in structures regulated under the IRC when an engineered design is submitted in accordance with Section R301.1.3 of the IRC.

3.0 PRODUCT DESCRIPTION

3.1 Product Information

3.1.1 CSH Concealed Stringer Hanger: The CSH-TZ stringer hanger provides a concealed connection for attaching the stair stringer to the carrying member. The CSH-TZ is fabricated from No. 18 gage steel and is prepunched for 10d-by-1½-inch nails into header, and 10d-by-1½-inch nails into the stair stringer. Table 1 and Figure 1 of this report list stock number, product dimensions, fastener schedule and allowable loads.

3.1.2 HDQIF Face Mount Inverted Flange Hangers: The HDQIF Face Mount Inverted Flange Hanger is designed to support headers, joists and trusses. The HDQIF Face Mount Hanger is cold-formed from No. 14 gage steel; and is pre-punched for ¹/₄-inch-by-3-inch USP WS3 wood screws into supporting member, and either ¹/₄-inch-by-1¹/₂inch USP WS15 wood screws or ¹/₄-inch-by-3-inch USP WS3 wood screws into the supported member. Table 2 and Figure 2 of this report list stock numbers, product dimensions, fastener schedule, and allowable loads.

3.1.3 HTC4 Heavy Truss Clip: The HTC4 Heavy Truss Clip provides lateral support for non-load bearing walls while allowing floor and roof trusses to deflect. The HTC4 is cold-formed from No. 16 gage steel; and is pre-punched for 10d-by-1-½-inch nails into plate and 10d-by-1-½-inch nails into the truss/rafter. Table 3 and Figure 3 of this report list stock numbers, product dimensions, fastener schedule and allowable loads.

3.1.4 JDS Purlin Hangers: The JDS Purlin Hanger is designed for purlin applications to provide double shear nailing. The JDS Purlin Hanger is cold-formed from No. 18 gage steel; and is pre-punched for 10d-by-1½-inch nails into header and 10d common nails into the joist. Table 4 and Figure 4 of this report list stock numbers, product dimensions, fastener schedule, and allowable loads.

3.1.5 JLIF Inverted Flange Joist Hangers: The JLIF Inverted Flange Joist Hanger is designed as a face-mount hanger to support nominal dimension joists. The JLIF is cold-formed from No. 18 gage steel; and is pre-punched for 10d or 16d common nails into the header and 10d-by-1-½-inch nails into the joist. Table 5 and Figure 5 of this report list stock numbers, product dimensions, fastener schedule, and allowable loads.

3.1.6 LJC Lateral Joist Connector: The LJC-TZ Lateral Joist Connector transfers lateral loads at the top foundation to the floor joists. The LJC-TZ is cold-formed from No. 18 gage steel and fastens the top side of the sill plate to the underside of the floor joist with 8d-by-1-½-inch nails. Table 6 and Figure 6 of this report list stock number, product dimensions, fastener schedule, and allowable loads.

3.1.7 LJQ Lateral Joist Connectors: The LJQ Lateral Joist Connectors transfers lateral loads at the top foundation to the floor joists. The LJQ Lateral Joist Connector is cold-formed from No. 16 gage steel; and is pre-punched for $\frac{1}{4}$ inch x 1- $\frac{1}{2}$ inch USP WS15 wood screws. Table 7 and Figure 7 of this report list stock numbers, product dimensions, fastener schedule, and allowable loads.



The product described in this Uniform Evaluation Service (UES) Report has been evaluated as an alternative material, design or method of construction in order to satisfy and comply with the intent of the provision of the code, as noted in this report, and for at least equivalence to that prescribed in the code in quality, strength, effectiveness, fire resistance, durability and safely, as applicable, in accordance with IBC Section 104.11. This document shall only be reproduced in its entirety.

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3.1.8 MUS Slant Nail Joist Hangers: The MUS Slant Nail Joist Hanger is designed for face-mount applications to provide double shear nailing for joist/truss-to-beam connections. The MUS Slant Nail Joist Hanger is coldformed from No. 18 gage steel and is prepunched for 10d common nails into both the header and the joist. Table 8 and Figure 8 of this report list stock numbers, product dimensions, fastener schedule, and allowable loads.

3.1.9 RBC Roof Boundary Clip: The RBC Roof Boundary Clip is designed to connect roof blocking to a wall top plate for a lumber connection having an angle ranging from 0 to 45 degrees. The RBC Roof Boundary Clip is cold-formed from No. 20 gage steel and is prepunched for 10d-by-1-½-inch nails into the top plate and 10d-by-1-½-inch nails into the blocking. Table 9 and Figure 9 of this report list stock numbers, product dimensions, fastener schedule, and allowable loads.

3.2 Materials

3.2.1 Steel: The specific types of steel and corrosion protection for each product are described in Table 10 of this report. Minimum steel base-metal thicknesses for the different gages are shown in the following table:

GAGE NO.	MINIMUM BASE-METAL THICKNESS (inch)
20	0.033
18	0.044
16	0.055
14	0.070

For **SI**: 1 inch = 25.4 mm.

3.2.2 Wood: Wood members shall be sawn lumber or structural glued laminated timber with a minimum specific gravity of 0.50, or approved structural engineered lumber (structural composite lumber, alternative strand lumber, or prefabricated wood I-joists) with a minimum equivalent specific gravity of 0.50, unless otherwise noted in the applicable table within this report. Wood members shall have a moisture content not exceeding 19 percent (16 percent for structural engineered lumber), except as noted in Section 4.1 of this report. For connectors installed with nails, the thickness of each wood member shall be sufficient such that the specified fasteners do not protrude through the opposite side of the member, unless otherwise permitted in the applicable table within this report. Wood members that are structural engineered lumber shall be recognized in, and used in accordance with, a current evaluation report prepared by an approved evaluation service agency. Section 3.2.5 of this report addresses uses related to treated wood.

3.2.3 Fasteners: Required fastener types and sizes for use with the USP structural connectors described in this report

are specified in this section and Tables 1 through 9 of this report.

3.2.4 Nails: Nails used for connectors described in this report shall comply with material requirements, physical properties, tolerances, workmanship, protective coating and finishes, and packaging and package marking requirements specified in ASTM F1667; and shall have lengths, diameters and bending yield strengths, F_{yb} , as shown in the following table:

1.5		
1.0	0.131	100,000
1.5	0.148	90,000
3.0	0.148	90,000
3.5	0.162	90,000
1.5	0.242	180,000
1.5	0.242	180,000
3.0	0.242	180,000
	3.0 3.5 1.5	3.00.1483.50.1621.50.2421.50.242

For SI: 1 inch = 25.4 mm, 1 psi = 6.89 kPa.

3.2.5 Use in Treated Wood: Connectors and fasteners used in contact with preservative-treated or fire-retardant-treated wood shall comply with Section 2304.9.5 of the 2012, 2009 and 2006 IBC, Section R317.3 of the 2012 and 2009 IRC, or Section R319.3 of the 2006 IRC. The lumber treater or the report holder (United Steel Products Company), or both, shall be contacted for recommendations on the appropriate level of corrosion resistance to specify for the connectors and fasteners as well as the connection capacities of the fasteners used with the specific proprietary preservative-treated or fire-retardant-treated wood.

4.0 DESIGN AND INSTALLATION

4.1 Design: The allowable load capacities in Tables 1 through 10 of this report are based on allowable stress design. The use of the allowable load values for the products described in this report shall comply with all applicable requirements and conditions specified in this report. Tabulated allowable loads are for normal duration or short duration, or both, based on load duration factors, C_D, in accordance with Section 11.3.2 of the AWC or AF&PA NDS. No further increases are permitted for load duration other than those specified in the tables. Tabulated allowable loads are for connections in wood seasoned to a maximum moisture content of 19 percent (16 percent for engineered wood products) or less, used under continuously dry conditions and where sustained temperatures are limited to 100°F (37.8°C) or less. When connectors are installed in wood having a moisture content greater than 19 percent (16 percent for engineered wood products), or where the in-service moisture content is expected to exceed this value, the applicable wet service factor, C_M, specified in Section 11.3.3 of the AWC or AF&PA NDS shall be applied. Unless otherwise noted in



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the tables of this report, the applicable wet service factor, C_M, is as specified in the AWC or AF&PA NDS for lateral loading of dowel-type fasteners. When connectors are installed in wood that will experience sustained exposure to temperatures exceeding 100° F (37.8°C), the allowable loads in this evaluation report shall be adjusted by the temperature factor, Ct, specified in Section 11.3.4 of the NDS. The group action factor, Cg, has been accounted for, in accordance with Section 11.3.6 of the NDS in the tabulated allowable loads, where applicable. The edge distances and end distances within the wood members shall be observed for connectors installed with bolts, such that the geometry factor, C_{Δ} , is 1.0, in accordance with Section 12.5.1 of the AWC or AF&PA NDS, unless otherwise in the report. Wood members shall be analyzed for loadcarrying capacity at connections in accordance with Section 11.1.2 of the AWC or AF&PA NDS.

For connectors intended to transmit loads into concrete or masonry by anchorage or bearing, design for anchor or bearing strength and anchor installation, including adequate embedment length and edge and end distances, shall be determined by a registered design professional in accordance with Chapter 19 or 21 of the IBC as applicable.

Where design load combinations include earthquake loads or effects, the design strength of anchorage to concrete shall be determined in accordance with Section 1909 of the 2012 IBC, or Section 1912 of the 2009 and 2006 IBC, except for detached one- and two-family dwellings, assigned to Seismic Design Category A, B or C, or located where the mapped short-period spectral response acceleration, S_s , is less than 0.4g.

4.2 Installation: Installation of the connectors shall be in accordance with this evaluation report and the manufacturer's published installation instructions. Where conflicts occur, the more restrictive shall govern. Mechanical fasteners shall be installed in wood members in accordance with AWC or AF&PA NDS Section 12.1. Figures 1 through 9 of this report provide installation details.

4.3 Special Inspection

4.3.1 IBC Wind: Periodic special inspection is required for installation of connectors described in this report that are designated as components within the main-wind-force-resisting-system of structures constructed when required in accordance with Sections 1704.2 and 1705.10 of the 2012 IBC, Section 1706.1 of the 2009 IBC and Section 1705.1 of the 2006 IBC. Special inspection requirements do not apply to structures, or portions thereof, that qualify for exception under the Sections 1704.1, 1704.4, 1706.2 or 1706.3 of the 2009 IBC and Section 1704.1 and 1704.4 of the 2006 IBC.

4.3.2 IBC Seismic: Periodic special inspection is required for installation of connectors described in this report that are designated as components of the seismic-force-resisting-system for structures in Seismic Design Category C, D, E or F in accordance with Sections 1704.2 and 1705.11 of the 2012 IBC, Sections 1707.3 or 1707.4 of the 2009 and 2006 IBC, with the exception of those structures that qualify under the Exceptions to Section 1704.1.

5.0 LIMITATIONS

The MiTek USP[®] structural connectors described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 if this report, subject to the following conditions:

5.1 The connectors shall be manufactured, identified and installed in accordance to this report and the manufacturer's published installation instructions. Where conflicts occur, the more restrictive shall govern. A copy of the manufacturer's published installation instructions shall be available at the jobsite at all times during installation.

5.2 Calculations showing compliance with this report shall be submitted to the code official. The calculations shall be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.

5.3 Connected wood members and fasteners shall comply with Sections 3.2.2 and 3.2.3 of this report, respectively.

5.4 Adjustment factors, noted in Section 4.1 of this report and the applicable codes, shall be considered in the design of the connections where applicable.

5.5 Use of connectors and fasteners with preservative-treated or fire-retardant-treated wood shall be in accordance with Section 3.2.5 of this report.

5.6 The design of the anchorage to, and bearing upon, concrete or masonry construction, inclusive of cast-inplace and post-installed anchors, used to attach the connectors described in this report to concrete or masonry construction shall comply with Section 4.1 of this report, and is outside of the scope of this report.

5.7 Structural members shall be checked for load carrying capacity at connections in accordance with Section 11.1.2 of the AWC or AF&PA NDS.

6.0 SUBSTANTIATING DATA

Testing and analysis data submitted is in compliance with ICC-ES Acceptance Criteria for Joist Hangers and Similar Devices (AC13), approved October 2016. Test results are from laboratories in compliance with ISO/IEC 17025.



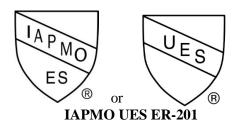
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7.0 IDENTIFICATION

Each connector described in this report is identified by the product model (stock) number, the number of the IAPMO index evaluation report for United Steel Products (ER-201) which identifies the products listed in this report, and by one or more of the following designations: USP or USP Structural Connectors, MiTek USA, Inc. Either UES Mark of Conformity may be used as shown below:



Brian Derben

Brian Gerber, P.E., S.E. Vice President, Technical Operations Uniform Evaluation Service

uchand

Richard Beck, PE, CBO, MCP Vice President, Uniform Evaluation Service

GP Russ Chaney CEO, The IAPMO Group

For additional information about this evaluation report please visit www.uniform-es.org or email at info@uniform-es.org



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TABLE 1: CSH-TZ CONCEALED STRINGER HANGER ALLOWABLE LOADS

STOCK	STEEL		DIMENSIO	NS (inches)		FASTENER	SCHEDULE		ALLOWABLE LOADS (lbs.)				
NUMBER	GAUGE	JOIST		HANGER		HEA	DER	JO	IST	DF-L/SP				
		w	w	Н	D	Qty	Туре	Qty	Туре	100%	115%	125%		
CSH-TZ	18	1 ¹ / ₂	1 ⁷ / ₁₆	5	5	8	10dx1-1/2	5	$10d \times 1^{1}/_{2}$	890	890	890		

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 6.89 kPa

 1A 10d x $1^{1}\!/_2$ nail is $1^{1}\!/_2$ inches long and 0.148 inches in diameter.

²The CSH-TZ is designed to support stair stringers and has not been torsionally tested for support of floor and roof framing members.

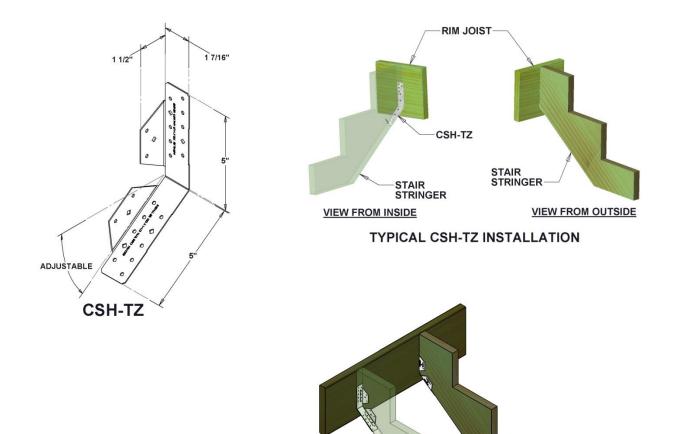


FIGURE 1 – CSH-TZ CONCEALED STRINGER HANGER



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TABLE 2: HDQIF FACE MOUNT INVERTED FLANGE HANGERS ALLOWABLE LOADS

STOCK	STEEL	DIME	ISIONS (i	inches)	FA	STENER S	SCHEDU	LE ¹²			ALLOW	ABLE LOA	DS (lbs) ³		
NO.	GA.	w	н	D	He	ader	Je	oist	F	c_ = 560 ps			c1 = 625 p	sl	Uplift
					Qty	Type	Qty	Туре	100%	115%	125%	100%	115%	125%	160%
HDQ179IF	14	1 ¹³ /16	9	3	8	WS3	4	WS15	3340	3605	3605	3340	3605	3605	1110
HDQ17112IF	14	1 ¹³ /16	11	3	10	WS3	6	WS15	3340	3340	3340	3340	3340	3340	1580
HDQ1714IF	14	1 ¹³ /16	13 ³ /a	3	12	WS3	6	WS15	4320	4525	4665	4660	4870	4955	2035
HDQ310IF	14	2 ⁹ /16	9	3	8	WS3	4	WS15	3340	3605	3605	3340	3605	3605	1110
HDQ210-2IF	14	31/4	9	3	12	WS3	6	WS3	5015	5590	5590	5015	5590	5590	2975
HDQ410IF	14	3 ⁹ /15	9	3	12	WS3	6	WS3	5015	5590	5590	5015	5590	5590	2975
HDQ412IF	14	3 ⁹ /16	11	3	14	WS3	6	WS3	5605	5605	5605	5605	5605	5605	3280
HDQ210-3IF	14	4 ⁵ / _n	9	3	12	WS3	6	WS3	5015	5590	5590	5015	5590	5590	2975
HDQ52101F	14	5 ¹ /4	9	3	12	WS3	6	WS3	5015	5590	5590	5015	5590	5590	2975
HDQ5212IF	14	5¼	11	3	14	WS3	6	WS3	5605	5605	5605	5605	5605	5605	3280
HDQ610IF	14	51/2	9	3	12	WS3	6	WS3	5015	5590	5590	5015	5590	5590	2975
HDQ612IF	14	51/2	11	3	14	WS3	6	WS3	5605	5605	5605	5605	5605	5605	3280

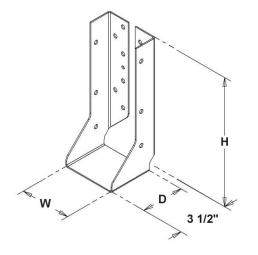
FOR SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 6.89 kPa

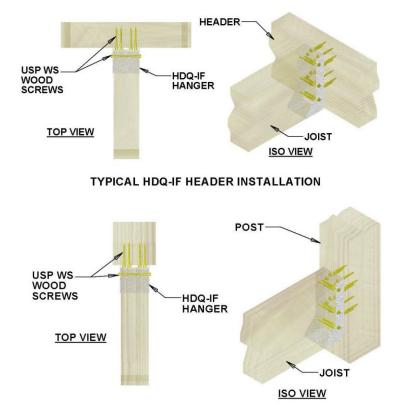
A WS15 is a 1/4" x 11/2" long self-drilling wood screw sold by USP and is included with each hanger.

² A WS3 is a ¹/₄ x 3⁻ long self-drilling wood screw sold by USP and is included with each hanger.

³ Loads have been increased as shown in accordance with code.

15% and 25% for snow and construction loads. No further increase is permitted. 60% for wind or seismic loading. No further increase is permitted.





TYPICAL HDQ-IF POST INSTALLATION

FIGURE 2 – HDQIF FACE MOUNT INVERTED FLANGE HANGERS



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TABLE 3: HTC HEAVY TRUSS CLIP ALLOWABLE LOADS

		DI	MENSI	ONS (ii	า.)		F	ASTENER SCH	IEDUL	_E ¹	ALLOWABLE LOADS (lbs.) ²			
STOCK	STEEL					Plate			Т	russ/Rafter	Withou	t Gap ³	With 1 ¹ / ₄ "Gap ⁴	
NO.	GA	w	н	L	D	Тор	Side	Туре	Qty	Turno	F1	F2	F1	F2
						Qty.	Qty.	Type	QLY	Туре	160%	160%	160%	160%
HTC4	16	3 ¹ / ₂	$3^{1}/_{4}$	1 ¹⁵ / ₁₆	1	2	4	4 10d x 1 ¹ / ₂ "		10d x 1 ¹ / ₂ "	255	525	55	295

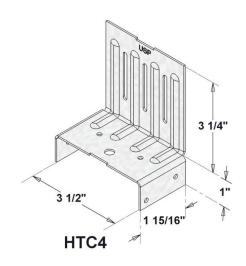
For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 6.89 kPa

 1 A 10d x $1^{1}\!/_{2}"$ nail has a diameter of 0.148 inches (No. 9 ga.) and length of 1 $^{1}\!/_{2}$ inches.

 $^2\text{C}_\text{D}$ is 60% for wind or seismic loads. No further increase shall be permitted.

³ Truss/Rafter must be bearing on top plate to achieve the allowable loads under "Without Gap"

 4 When installed with maximum $1^{1/4}$ inch space between truss/rafter and top plate, use loads under "With $1^{1/4}$ " Gap"



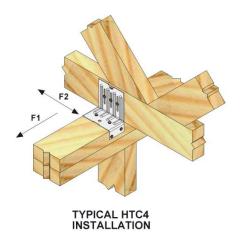


FIGURE 3 – HTC HEAVY TRUSS CLIPS



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070.01/					,							10				2.4
STOCK NO.	STEEL GA.	DIN	ENSIO	NS (inch	es)	PUR	LIN(S)		FASI	ENER SCH	EDULE	1,2	A	LLOWABLE	LOADS (lbs)	3,4
NO.	GA.	w	Н	D	TF	Qty	Size		Head	er		Joist		Fc = 625 ps	si	Uplift
								Тор	Face	Туре	Qtv	Туре	C _D =1.0	C _D =1.15	C _D =1.25	C _D =1.6
JDS24S	18	1 _{9/16}	3 ½	1 ¼	3⁄4	1	2 x 4	2	0	10dx1½	2	10d Common	575	575	575	340
JDS24	18	1 _{9/16}	3 ½	1 ¼	1 _{9/16}	2	2 x 4	4	0	10dx1½	2	10d Common	1,155 ⁵	1,155 5	1,155 ^₅	680 ⁵
JDS26S	18	1 _{9/16}	5 ½	1 ¼	3⁄4	1	2 x 6	2	0	10dx1½	4	10d Common	775	830	835	390
JDS26	18	1 _{9/16}	5 ½	1 ¼	1 _{9/16}	2	2 x 6	4	0	10dx1½	4	10d Common	1,575 ^₅	1,670 ⁵	1,670 ^₅	775 ⁵

TABLE 4 - JDS PURLIN HANGER SERIES

For SI: 1 inch =25.4 mm, 1 lbf =4.45 N, 1 psi =6.89 kPa

 $^{\scriptscriptstyle I}$ A 10d x 1½ inch nails have a diameter of 0.148 inch and a length of 1½ inches.

²A 10d common nail has a diameter of 0.148 inch and a length of 3 inches.

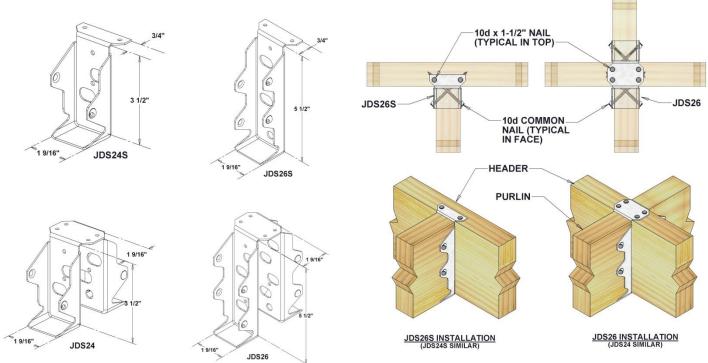
³The listed loads assume the supporting member is not less than 1 1/2 inches thick.

⁴Loads have been increased as shown in accordance with the AWC or AF&PA NDS.

 C_D is 1.15 and 1.25 for snow and construction loads, respectively. No further increase is permitted.

 C_D is 1.60 for wind or seismic loading. No further increase is permitted.

⁵ Allowable loads are for both sides combined. No one side shall resist more than one-half the tabulated load.



TYPICAL JDS PURLIN HANGER INSTALLATION





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TABLE 5: JL-IF INVERTED FLANGE HANGER SERIES

						FASTENER S	CHEDUL	E ¹²³	Α	LLOWABLE	LOADS (lbs.	.) ⁴
STOCK	STEEL	DIME	ENSIONS	i (in.)		Header		Joist		DF-	_/SP	
NUMBER	GAGE				Nail	Nail	Nail	Nail	100%	115%	125%	Uplift
		W	Н	D	Qty	Туре	Qty	Туре	100%	11370	123%	160%
JL24-IF	18	1 ⁹ / ₁₆	3 ¹ / ₈	1 ¹ / ₂	4	10d Common	2	10dx1 ['] / ₂	465	535	580	280
JL24-IF	10	1 / ₁₆	3/8	1/2	4	16d Common	2	$10dx1^{1}/_{2}$	550	615	615	280
JL26-IF	18	1 ⁹ / ₁₆	4 ¹ / ₂	1 ¹ / ₂	6	10d Common	4	10dx1 ['] / ₂	695	800	870	730
JL20-IF	10	1 / ₁₆	4 /2	1/2	6	16d Common	4	10dx1 ¹ / ₂	830	950	1035	730
JL28-IF	18	1 ⁹ / ₁₆	6 ¹ / ₈	$1^{1}/_{2}$	8	10d Common	4	10dx1 ['] / ₂	930	1065	1160	730
JL20-IF	10	1 / ₁₆	ю/ ₈	1/2	8	16d Common	4	$10dx1^{1}/_{2}$	1105	1215	1215	730
II 210 IE	10	49/	8 ¹ / ₄	4 1/	11	10d Common	6	10dx1 ['] / ₂	1275	1465	1595	1095
JL210-IF	JL210-IF 18 1 ⁹ / ₁	1 / ₁₆	8/4	1 ¹ / ₂	11	16d Common	6	$10dx1^{1}/_{2}$	1520	1745	1900	1095

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 6.89 kPa

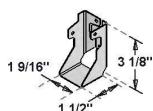
¹ A 10d Common nail is 0.148 inches in diameter and 3 inches long

 2 A 16d Common nail is 0.162 inches in diameter and $3^{1}\!/_{2}"$ inches long

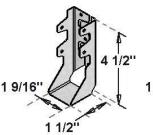
 3 A 10dx1 $^{1}/_{2}$ " nail is 0.148 inches in diameter and 1 $^{1}/_{2}$ " inches long

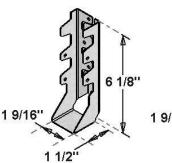
⁴ Loads have been increased as shown in accordance with code.

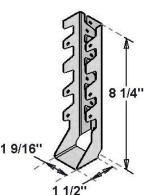
15% and 25% for snow and construction loads. No further increase is permitted.60% for wind or seismic loading. No further increase is permitted.



JL24-IF







JL210-IF

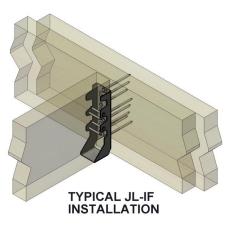


FIGURE 5 – JLIF-TZ INVERTED FLANGE JOIST HANGERS



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TABLE 6: LJC-TZ LATERAL JOIST C	CONNECTOR ALLOWABLE LOADS
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STOCK	STEEL		FASTENER	SCHE	DULE	DF-L/SP ALLOWABLE LOADS (lbs.) ¹				
NUMBER	GAGE	LJC	C to Joist	LJO	C to Plate	Compression				
HOMBER	ő	Qty	Type ²	Qty	Type ²	90%	100%	160%		
LJC-TZ	18	6 8d x 1 ¹ / ₂		6	8d x 1 ¹ / ₂	515	570	670		
Fan Cl. 4 in als	05 4	4 11-5	445 N 4 -	-: C	00 1-D-					

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 6.98 kPa

¹ Loads have been decreased or increased as show n in accordance with code.

 C_D is 90% for earth loading

 $C_{\!D}$ is 160% for wind or seismic loading. No further increase is permitted.

 2 A 8d x 1-1/2 nail has a diameter of 0.131 inch and a length of 1- $^1\!/_2$ inches.

³ All nails must be installed with sufficient edge distances to prevent splitting of member.

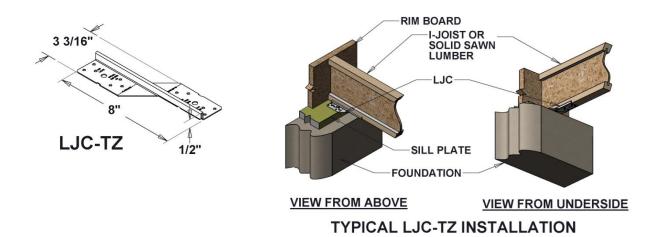


FIGURE 6 – LJC-TZ LATERAL JOIST CONNECTOR



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TABLE 7: LJQ-TZ LATERAL JOIST CONNECTOR ALLOWABLE LOADS

STOCK	STEEL	DIME	NSIONS (ir	nches)	I	FASTENE	R SCHED	ULE	SOUTHERN PINE SILL PLATE				
NUMBERS	GAGE	Width	Length	Depth	LJQ to	o Joist	LJQ	to Plate	ALLOWABLE COMPRESSION				
	OAGE	(in.)	(in.)	(in.)	Qty	Туре	Qty	Type ²	90%	1 00 %	160%		
LJQ15-TZ	16	1 ⁹ / ₁₆	3	$1^{1}/_{2}$			4	WS15-GC	915	1015	1110		
LJQ17-TZ	16	1 ¹³ / ₁₆	3	$1^{1}/_{2}$			4	WS15-GC	915	1015	1110		
LJQ20-TZ	16	2 ¹ / ₈	3	1 ¹ / ₂			4	WS15-GC	915	1015	1110		
LJQ23-TZ	16	2 ⁵ / ₁₆	3	1 ¹ / ₂			4	WS15-GC	915	1015	1110		
LJQ25-TZ	16	2 ⁹ / ₁₆	3	1 ¹ / ₂			4	WS15-GC	915	1015	1110		
LJQ35-TZ	16	3 ⁹ / ₁₆	3	1 ¹ / ₂			4	WS15-GC	915	1015	1260		

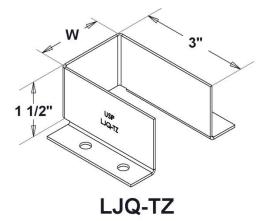
For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 6.98 kPa

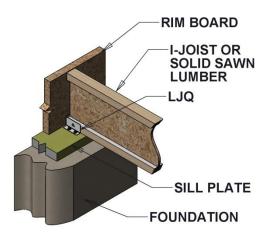
¹ Loads have been decreased or increased as show n in accordance with the NDS.

 $C_{\!\scriptscriptstyle D}$ is 90% for earth loading

 C_{D} is 160% for wind or seismic loading. No further increase is permitted.

 2 WS15-GC wood screws are $^{1}\!/_{4}^{"}$ diameter x 1- $^{1}\!/_{2}^{"}$ long and are included with connector.





VIEW FROM ABOVE

TYPICAL LJQ-TZ INSTALLATION

FIGURE 7 – LJQ-TZ LATERAL JOIST CONNECTOR



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TABLE 8: MUS SLANT NAIL JOIST HANGER ALLOWABLE LOADS

		D	IMENSI	ONS (ir	າ.)		FASTENER S	CHEDU	JLE ¹²	DF-L/S	P ALLOWA		S (lbs.)
STOCK	STEEL	×	1	C	~		Header	Joist		0	Downward	3	Uplift ³
NUMBER	GA.	vv		D	~	Qty	Туре	Qty	Туре	100%	115%	125%	160%
MUS26	18	1 ⁹ / ₁₆	5 ¹ / ₁₆	2	1	6	10d Common	6	10d Common	1285	1475	1605	865
MUS28	18	1 ⁹ / ₁₆	7 ¹ / ₁₆	2	1	8 10d Common		8	10d Common	1710	1970	2140	1230

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 6.89 kPa

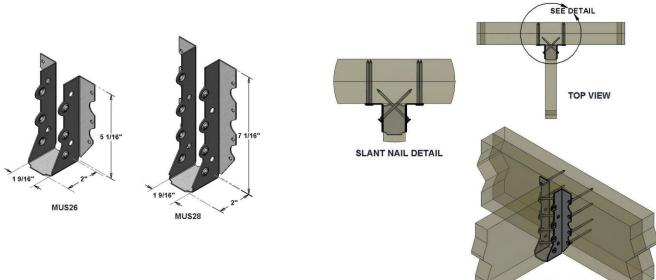
¹ A 10d Common nail is 3 inches long and 0.148 inches in diameter.

² The 10d Common nails driven into the joist shall be installed at 30 to 45 degrees horizontally toward the header.

³ Loads have been increased as shown in accordance with the NDS.

 C_{D} is 115% and 125% for snow and construction loads. No further increase is permitted.

 C_{D} is 160% for wind or seismic loading. No further increase is permitted.



TYPICAL MUS

FIGURE 8 – MUS SLANT NAIL JOIST HANGERS

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TABLE 9: RBC ROOF BOUNDARY C	CLIP ALLOWABLE LOADS
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STOCK	STEEL	INSTALLATION	FASTENER SCHEDULE ²			Load ⁴	ALLC	WABLE	LOADS (I	bs.) ³	
NUMBER	GAGE	CONFIGURATION	-	Header / Stud Joist / Plate		Direction	100%	115%	125%	160%	
RBC	20	Wood to Wood	6	10d x 1 ¹ / ₂ "	6	10d x 1 ¹ / ₂ "	F1	510	510	510	510
RBC	20	Wood to CMU	3	¹ / ₄ " x 2 ¹ / ₄ " Tapcon	6	$10d \times 1^{1}/_{2}$ "	F1	450	450	450	450
FOR SI: 1 inch = 25.4 mm, 1lbf = 4.45 N, 1 psi = 6.89 kPa											

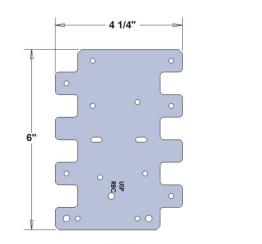
¹ Refer to drawings for installation configuration.

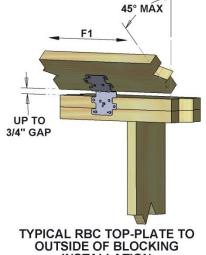
 2 A 10d x 1 $^{1}/_{2}$ " nail has a diameter of 0.148 inches and length of 1 $^{1}/_{2}$ inches.

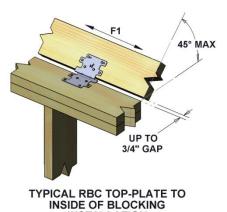
 $^{1}/_{4}$ " x $2^{1}/_{4}$ " Tapcons are manufactured by ITW-Buildex.

³ Allowable loads have been increased in accordance with the NDS: C_D is 115% for snow load, 125% for construction loads and 160% for wind or seismic loads. No further increase is permitted.

⁴ Refer to installation drawings for the load direction.

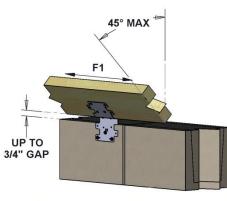






INSTALLATION





INSTALLATION

TYPICAL RBC CONCRETE BLOCK WALL TO BLOCKING INSTALLATION



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TABLE 10: STEEL, TYPE, STRENGTH, AND CORROSION RESISTANCE

PRODUCT	STEEL ²	COATING ¹
CSH-TZ Concealed Stringer Hanger	18 Gage ASTM A653 SS Grade 40	G185
HDQIF Face Mount Inverted Flange Hanger	14 Gage ASTM A653 SS Grade 40	G90
HTC4 Heavy Truss Clip	16 Gage ASTM A653 SS Grade 40	G90
JDS Purlin Hanger	18 Gage ASTM A653 SS Grade 40	G90
JLIF Inverted Flange Joist Hanger	18 Gage ASTM A653 SS Grade 40	G185
LJC Lateral Joist Connector	18 Gage ASTM A653 SS Grade 40	G185
LJQ Lateral Joist Connector	16 Gage ASTM A653 SS Grade 40	G185
MUS Slant Nail Joist Hanger	18 Gage ASTM A653 SS Grade 40	G90
RBC Roof Boundary Clip	20 Gage ASTM A653 SS Grade 40	G90

¹ Corrosion protection is a zinc coating of sheet steel in accordance to ASTM A653.

 2 Steel designated as ASTM A653 SS Grade 40 in the table above has a specified minimum yield strength, F_y, of 40 ksi (276 MPa), and a specified minimum tensile strength, F_u, of 55 ksi (379 MPa).